

**UNIVERSITI TEKNOLOGI MARA**

**INVESTIGATION ON THE  
PERFORMANCE OF GFRP  
REINFORCED GLUED LAMINATED  
TIMBER (GLULAM COMPONENT)**

**ABDULLAH OMAR BIN ABDULLAH ZAMLI**

Thesis submitted in fulfilment  
of the requirements for the degree of  
**Master of Science**  
**(Civil Engineering)**

**Faculty of Civil Engineering**

**May 2019**

## ABSTRACT

The glulam method enabled the use of smaller timber pieces being formed together into a stronger larger piece, a revolutionary method which made timber construction component much more reliable renewable material. However this method still relies on the rule that a larger component is always stronger. Several applications reinforcement like glass fibre reinforced polymer (GFRP) or carbon fibre reinforced polymer (CFRP) have been applied on glulam in order to allow it to have greater structural strength without any increase of its dimensions. Though reinforcement is proven to be workable, there is still no standardized method of manufacture and minimum parameter of reinforcement materials. Also there are no in depths understanding of an optimized alignment of the reinforcement fibres in relation to the glulam wood grain. This study had done flexural tests using solid timber with dimension of 50mm x 30mm x 3300mm and glulam beams with dimension of 140mm x 80mm x 3000mm. It has been identified from the solid timber test that the direction of GFRP strips at diagonal ( $45^\circ$ ) to the wood grain of a solid timber, gives 4.05% increase of the load carrying capacity compared to control samples while reinforcement perpendicular ( $90^\circ$ ) to the wood grain had an insignificant increase if at all. This justified in the glulam test to place the GFRP strips between the fifth and fourth layers of lamination from the top, at angle close to parallel to the wood grain (zero direction) and tested under a 4-point flexural test. The glulam beam flexural tests with varied area of 30%, 40% and 50% area of reinforcement over the lamination surface have an average maximum load capacity of 49.40kN, 45.81kN and 50.39kN respectively, compared with the control samples that have average maximum load at 54.69kN. This clearly shows that the reinforced test samples with varied area of 30%, 40% and 50% area of reinforcement each have a decrement in maximum load capacity at -9.67%, -16.24% and -7.86% respectively and decrement of modulus of rupture with -9.67%, -16.23% and -7.85% each respectively. While analysis of the damage pattern of the glulam beam and GFRP strips indicated a severe effect of delamination due to the difference of material behaviour when placed under load. These findings indicate that the alignment and layering of the GFRP strips could seriously affect the glulam strength in a detrimental manner, hinting and underlying mechanics at work. More study should be done to understand the alignment design to prevent premature delamination between reinforcement with the timber component that may lead to a terminal structural failure.

## ACKNOWLEDGEMENT

Firstly, I wish to thank God for giving me the opportunity to embark on my Master of Science study and for completing this long and challenging journey successfully. My gratitude and thanks go to my supervisor Assoc. Prof. Dr. Rohana Hassan, and co-supervisor, Dr. Anizahyati Alisibramulisi. Thank you for the support, patience and ideas in assisting me with this project.

I also would like to express my gratitude to the staff of the UiTM FCE Lecturers, laboratory technical staffs and MyGlam Sdn. Bhd., especially En. Mohd Yasin, En. Asmadi and En. Lim for providing the facilities, knowledge and assistance. Special thanks to my colleagues and friends for helping me with this project.

Finally, this thesis is dedicated to my caring and supportive parents [REDACTED] [REDACTED] En. Abdullah Zamli B. Alias, for the vision and determination to educate me and see me through till the end. This piece of victory is dedicated to both of you. Alhamdulillah.

# TABLE OF CONTENTS

	<b>Page</b>
<b>CONFIRMATION BY PANEL OF EXAMINERS</b>	<b>ii</b>
<b>AUTHOR'S DECLARATION</b>	<b>iii</b>
<b>ABSTRACT</b>	<b>iv</b>
<b>ACKNOWLEDGMENT</b>	<b>v</b>
<b>TABLE OF CONTENTS</b>	<b>vi</b>
<b>LIST OF TABLES</b>	<b>ix</b>
<b>LIST OF FIGURES</b>	<b>xi</b>
<b>LIST OF SYMBOLS</b>	<b>xvi</b>
<b>LIST OF ABBREVIATIONS</b>	<b>xvii</b>
<b>LIST OF NOMENCLATURES</b>	<b>xviii</b>
<b>CHAPTER ONE: INTRODUCTION</b>	<b>1</b>
1.1 Research Background	1
1.2 Problem Statement	2
1.3 Objectives	3
1.4 Scope of Research	3
1.5 Significance of Study	5
<b>CHAPTER TWO: LITERATURE REVIEW</b>	<b>6</b>
2.1 Introduction	6
2.2 Timber Selection	7
2.3 Moisture Content	12
2.4 Bending Strength of Timber Species	13
2.5 Glued Laminated Timber	16
2.6 Glass Fibre Reinforced Polymer (GFRP)	19
2.7 GFRP for Timber Repairs	24
2.8 GFRP as Timber Reinforcement	30
2.9 Variations of GFRP Alignment & Applied Surface Area	33
2.10 Adhesive Component	36

2.11	Summary / Research Gap	39
<b>CHAPTER THREE: METHODOLOGY</b>		<b>40</b>
3.1	Introduction	40
3.1.1	Material	41
3.1.2	Timber Species	42
3.1.3	Treatment & Curing	43
3.1.4	Lumber Surfacing	44
3.1.5	Bonding Adhesive	44
3.1.6	Glass Fibre Reinforced Polymer (GFRP) Strips	46
3.1.7	Instalment of GFRP Strips	46
3.2	Solid Timber Test	48
3.2.1	Specimens Preparation	49
3.2.2	Flexural Test	50
3.2.3	Moisture Content	52
3.3	Glulam Beam Test	53
3.3.1	Component Design Details	54
3.3.2	Flexural Test	56
3.3.3	Delamination Test	58
3.3.4	Moisture Content	61
3.4	Summary	61
<b>CHAPTER FOUR: ANALYSIS OF RESULTS</b>		<b>62</b>
4.1	Introduction	62
4.2	Solid Timber Test Results Analysis	62
4.2.1	Failure of Yellow Meranti and Bintangor Control Specimens	63
4.2.2	Failure pattern of Yellow Meranti and Bintangor specimens with Perpendicular (90°) and Diagonal (45°) to the wood grain GFRP strips Alignment	64
4.2.3	Load Capacity of Small Scale Yellow Meranti and Bintangor Specimens	67
4.2.4	Modulus of Elasticity of small scale Yellow Meranti and Bintangor specimens	69