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CURVI-LINEAR SHEAR STRENGTH ENVELOPE FOR RESIDUAL SOIL IN PRECINCT 9, PUTRAJAYA FOR UNSOAKED AND SOAKED MULTISTAGE TRIAXIAL DRAINED TEST

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Abstract

To determine the mode of slope failures is very difficult especially when using the conventional slope stability method which applies the linear type of strength variation with respect to effective stress and suction. These studies were conducted to fulfil the real failure mechanism of slope failure which included the analysis of shear strength behaviour incorporating curvi-linear relative to effective stress and non-linear relative to suction. Three undisturbed samples were collected at Precinct 9, Putrajaya at different depths to determine the multistage triaxial drained tests with unsoaked, soaked for 7 days and soaked for 14 days samples. Findings show that from the test, there were slight reductions in shear strength between all the samples. For unsoaked sample, the shear strength was 28°, , soaked for 7 days was 25° and for soaked for 14 days was 22°. The shear strength parameters obtained from the results will be used in slope stability analysis in order to define the Factor of Safety for slope failure.

Keywords: Multistages, Shear Strength, Unsoaked, Soaked

1. Introduction

Malaysia is located in a semi arid region which has hot and humid climate all year long. The pattern of slope failure that occurred normally of shallow types and the groundwater table were usually very deep and had no effect on the failure. This is a common type of failure in tropical countries which received high annual rainfall. The term “shallow” here is referring to the movement of the soil above the toe which moves downwards without under cutting the toe. Slope failure of shallow type is confined within unsaturated zone when it is infiltrated by the surface water. It is very important to understand the mode of failure triggered by rain water infiltration. Despite the soil condition during failure is unsaturated, the shear strength applied in a slope stability analysis is always the saturated shear strength since in the conventional triaxial test the specimen was already being saturated prior to shearing

Hence, the understanding of mode of failure in natural or man-made slope is required in order to produce an appropriate slope stability equation where it is applicable to real case scenario of a slope failure. By understanding the actual mode of failure occurs in shallow type of landslide, it is hoped that the slope stability computational programme will be more reliable and reflective of the actual stability state. Furthermore, during the stage of design, most of the geotechnical engineer’s tend to design the slope stability based on empirical methods which require a high degree of engineering judgment which are derived from a lot of practical experiences.

2. Literature Review

Shear strength can be defined as the maximum value of shear stress that the soil can withstand. The shear strength of soils is controlled by effective stress, weather failure occurs under drained or undrained conditions (Duncan and Wright, 2005). The first ever shear strength equation that incorporates with a linear shear strength envelope with respect to effective stress was introduced by Terzaghi in 1936. After that, in 1960, Bishop et. al. extend the understanding of Terzaghi equation by introduced the relationship between the degree of saturation, S_r and the empirical parameter, χ . The value of χ is assumed to vary from 1 to 0, which represents the variation from a fully saturated condition to a total dry condition. These studies have indirectly provided an important

milestone in understanding the relationship between shear strength and the soil-water characteristic curve behaviour. (Barbour, 1999) where the stress that sustain by the skeleton is known as effective stress. The hydrostatic stress that exists from the water in the voids is known as pore water pressure.

In practice, geotechnical engineers always assume that the pore water pressure above the ground water table is negligible and in a positive values whereas in reality, the unsaturated soil which above the ground water table, the pore water pressure is in a negative values. Pore water pressure of negative values with respect to the atmospheric pressure contributes to a higher effective stress and thus in a higher shear strength. (Md. Noor, 2002). Furthermore, for an effective stress analysis, a soil is assumed to be in a drained condition whereas the changes in load are slow enough that the load induces no excess pore water pressure and the water can slowly dissipate from soil. Therefore, to perform effective stress analysis, it is necessary to subtract the pore water pressure from the total stress (Duncan and Wright, 2005).

The first equation that incorporates the two independent stress state variables which net stress ($\sigma - u_a$) and suction ($u_a - u_w$) are commonly used to describe the behaviour of unsaturated soil is proposed as a planar surface in $\tau:(\sigma - u_a):(u_a - u_w)$ space, which refers to extended Mohr Coulomb space. (Fredlund et al., 1978). Referring to Figure 1, the figure shows the failure envelope drawn tangent to the Mohr circle. The shear strength of unsaturated soil is drawn based on the extended Mohr Coulomb failure envelope.

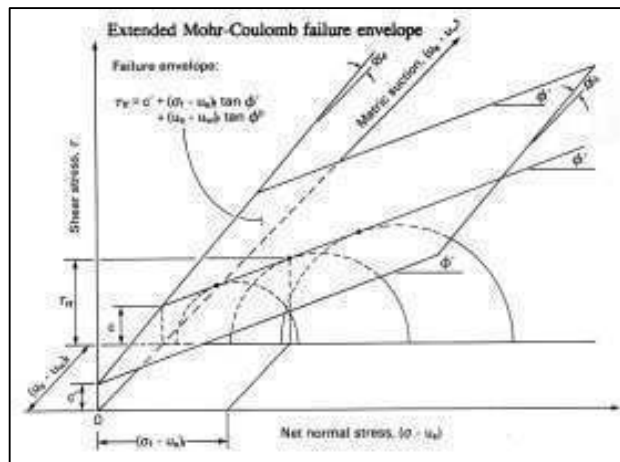


Figure 1: Shear strength model of Fredlund et al., (1978)

$$\tau_f = c' + (\sigma_n - u_a) \tan \phi' + (u_a - u_w) \tan \phi^b$$

Where,

- σ_n = total normal stress
- u_a = pore-air pressure
- u_w = pore-water pressure
- ϕ^b = rate of increase in shear strength with matric suction or unsaturated friction angle

The shear strength of an unsaturated soil is considered to consist of an effective cohesion, c' and the independent contributions of net normal stress, $(\sigma - u_a)$ and the matric suction, $(u_a - u_w)$. The shear strength contributed from net normal stress and matric suction are characterized by ϕ' and ϕ^b angles respectively. The equation of this shear strength for unsaturated soil condition is considered linear with respect to effective or net stress whereas recent experimental evidence established by Donald, 1956; Escario and Saez, 1986; and Fredlund, Rahadrjo and Gan, 1987; suggest that there can be a significant non-linearity in the shape of failure envelope with respect to matric suction. Figure 2 shows the non-linear behaviour of shear strength with respect to suction which done by Gan and Fredlund, 1996. The figure shows the experimental evidence of the non-linear behaviour due to suction in different value of matric suction.

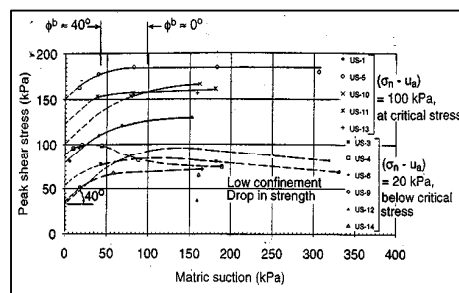


Figure 2: Non-linear behaviour of shear strength with suction. (Gan and Fredlund, 1996)

Bishop, 1966 conducted shear strength test for different soil properties shown in Figure 3. The testing was carried out for two different soil categories namely granular and cohesive soils. All the result shows that the shear strength was intercept at zero in shear stress-effective stress plane.

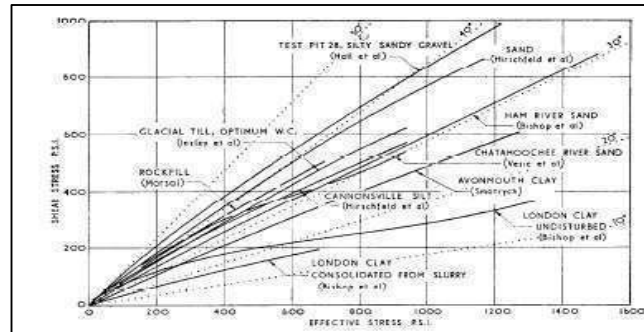


Figure 3: Non-linear shear strength behaviour with respect to effective stress (Bishop, 1966)

Furthermore, substantial amount of experimental evidence suggest that the failure criteria of many soils are non-linear, particularly in a range of small normal stresses. (Penman, 1953, Day and Axten, 1989). This finding of non-linearity in Mohr Coulomb failure envelope is significant for slope stability calculation since for a wide range of practical stability problems, critical slip surface is shallow and normal stresses acting on such surfaces are small. A comprehensive approach to a slope stability calculation which integrates experimental and computational aspects of the problem, shows that non-linearity of failure criterion may have a very significant effect on slope design. (Baker, 2003).

3. Research Methodology

The determination of engineering properties will based on finding the effective cohesion, c' and effective internal friction angle, ϕ' of the samples. Therefore, in order to determine this parameter, triaxial testing will be carried out. Triaxial testing is the common method that use to determine the shear strength of the soil. Among the advantages of using this triaxial is that the shear failure plane is not predetermine but the shear failure plane will occur at the weakest spot of the sample. Besides that, the excessive of pore water pressure can be controlled, monitored and measured. In addition, with these aid of excessive pore water pressure, the sample can be strengthen first by means of consolidated the specimen. During this stage, the axial load will load slowly enough so that no access of pore water pressure is induced by applied loads. The relation on the field is when in drained condition result when loads are applied slowly to a mass of soil, or where they persists for a long enough time so that the soil can drain. In the laboratory, drained conditions are achieved by loading the sample slowly so that excess pore pressure does not develop as the soil is loaded. The shear strength parameters gathered are the effective internal friction angle, ϕ' , transition shear strength, τ_t and transition effective stress, $(\sigma - u_w)_t$ according to the curved-surface envelope shear strength model. The shear strength properties will be interpreted based on the curved-surface envelope shear strength model of Md. Noor and Anderson (2006) and linear envelope of Terzaghi (1936) and Fredlund et al. (1978).

In addition, multi-stage consolidated drained triaxial was decided in order to avoid the problem of variability between samples. When this has been avoided it will ease the interpretation of the shear strength envelope. This testing technique is very important when the effect of soaking on the reduction of shear strength is to be incorporated in the analysis. Variability between the samples will cause the complexity in the interpretation of strength envelope itself and further complexity when the effect of soaking is to be incorporated. The shearing for the multi-stage test is terminated when failure is imminent. About 3 samples in different condition of test will be carried out. For unsoaked condition, the sample was tested in normal ways of triaxial testing which being consolidated in the first place and water allow to flow through the sample prior to shearing stages while for soaking condition weather for 7 or 14 days, the sample was soaked first before it can be consolidated and shear. The soaking process can be achieved in cell bodies by allowing back pressure and cell pressure flowing through which a value of 50 kPa will be applied to the cell pressure and 45 kPa will be applied to back pressure. Thus, the different of 5 kPa will be the soaking value of the sample allowing through for the desired time. Sample will be under soaking process for the desired time where all the microstructures of the sample are being saturated.

Table 1: Tabulation of multistage consolidated drained test in Precinct 9, Putrajaya

Location	Borehole No.	Depth (m)	Condition of Samples	Effective stress (kPa) CD triaxial test			
				50	100	200	300
Precinct 9	BH1	1.5	Unsoaked	50	100	200	300
	BH2	4.0	Soaked for 7 days	50	-	200	300
		5.0	Soaked for 14 days	50	100	200	300

Table 1 shows the tabulation of multistage consolidated drained test in Precinct 9, Putrajaya. There are about 3 conditions of samples which are unsoaked, soaked for 7 and 14 days were tested. These undisturbed samples were tested at different of effective stress ranging from 50 to 300kPa while Figure 4 shows the layout location of Borehole 1 and Borehole 2 in Precinct 9, Putrajaya. About 3 undisturbed samples were used for the test at different depth where for unsaturated condition, the samples were taken at the depth of 1.5m from BH1. On the other hand, samples that used for the soaked test in 7 and 14 days were collected at the depth of 4 and 5m consecutively from BH2.

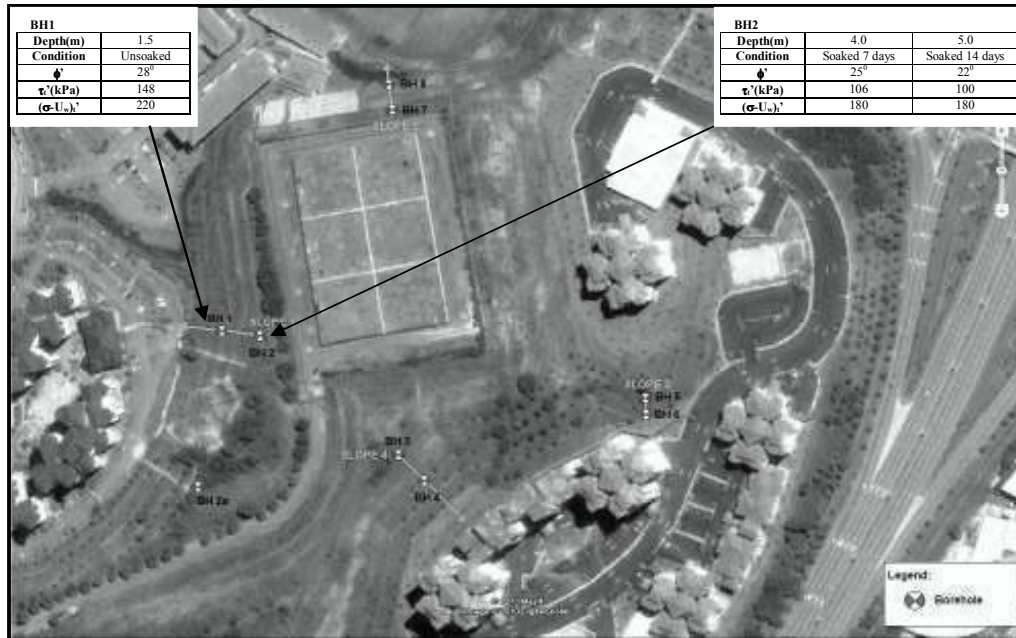


Figure 4: Layout location of borehole for Precinct 9, Putrajaya

4. Result and Analysis

Table 2 shows the tabulation of effective shear strength parameters incorporates curvi-linear shear strength envelope for Precinct 9, Putrajaya. Finding shows that there are slightly decreasing in shear strength between unsoaked and soaking samples. For unsoaked sample, the effective internal friction angle, ϕ' was 28° while for the samples that been soaked for 7 and 14 days given values of friction angle about 25° and 22° consecutively. Its mean that there is a significant reduction in term of effective friction angle for unsoaked sample compared to soaked for 7 days by 3° while for sample been soaked for 14 days, the effective friction angle keep decreasing by 6° compare to unsoaked sample.

Table 2: Tabulation of effective shear strength parameters incorporates curvi-linear shear strength envelope for Precinct 9, Putrajaya

Location	Borehole No.	Depth (m)	Effective Stress, kPa	Condition of Failure			Shear Strength Parameters		
				DS (kPa)	PWP (kPa)	CP (kPa)	ϕ'	τ_t kPa	$(\sigma - U_w)_t$ kPa
Precinct 9	BH1 Unsoaked	1.5	50	161	386	450	28°	148	220
			100	266	386	498			
			200	416	385	595			
			300	565	385	695			
	BH2 Soaked	4.0 (7 days)	50	134	483	544	25°	106	180
			200	220	496	690			
			300	472	512	789			
		5.0 (14 days)	50	165	437	494	22°	100	180
			100	229	437	546			
			200	311	437	646			
			300	444	437	746			

In order to determine the effective internal friction angle, ϕ' of the sample, stress-strain plain needs to be plotted using data in multistage triaxial testing. The maximum stress of the sample when failure will be recorded in each stage of effective stress as shows in Figure 5.

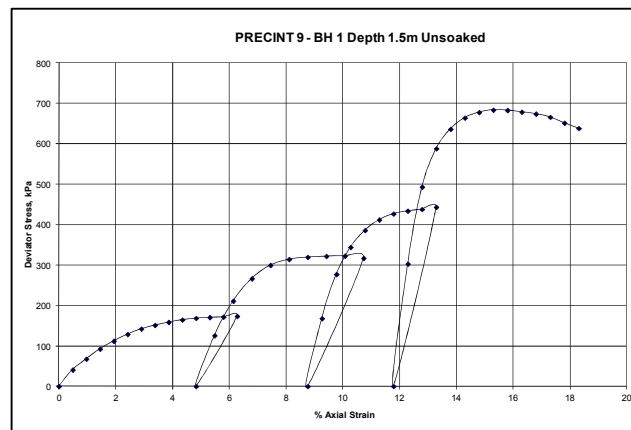


Figure 5: Multistage stress-strain for unsoaked sample depth of 1.5m in Precinct 9, Putrajaya

Figure 6 shows the combination of failure envelope for each sample in different condition when using curvi-linear shear strength envelope. It found that the shear strength was reduced when soaked into the water for 7 and 14 days consecutively compared to unsoaked sample. The reduction of shear strength between soaked 7 and 14 days were at 3° while for unsoaked compare to soaked for 14 days were about 6°. Finding shows that during soaking period, the micro structures of the soil is getting weaker due to existence of water. Upon that, if the soil is being wetted for a longer time, the shear strength of the soil will keep decreasing.

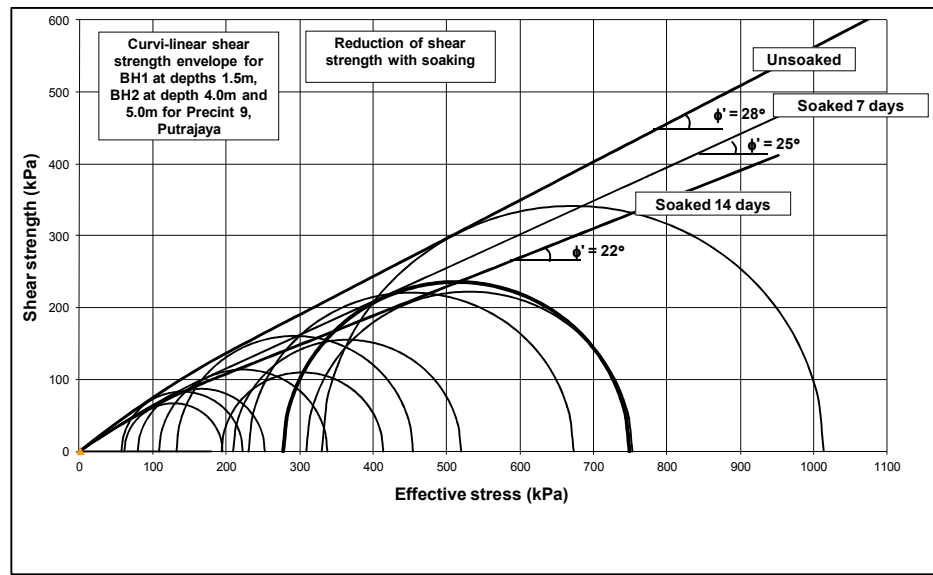


Figure 6: Combination failure envelope using curvi-linear shear strength envelope for unsoaked, soaked for 7 days and soaked for 14 days.

5. Conclusion

There is a significant reduction between unsoaked and soaked samples with respect to effective friction angle, ϕ' of the shear strength of the soil. If the soil is been soaked into water, the shear strength will be decreasing tremendously and it will further decrease with time when the soil is too long soaked into water. These can be found from the results where there are substantial different amount of effective friction angle by 6° when compare to unsoaked and soaked sample for 14 days. Furthermore, there is also a reduction in effective friction angle by 3° between unsoaked and soaked sample for 7 days. Soaking sample in the triaxial involved complex processes and it is a kindly new approached to monitor the effect of shear strength of soil due to soaking effect. Findings have proven that shear strength in soil will decline when the soil is soaked with water and will keep declining with time especially when it involved with clayey soil. Slope failure are normally associated with the rise of ground water table but in this research found that the most reasonable reason why slope failure are because of soil is being soaked into water for too long by water infiltration from rainfall intensity.

As a recommendation, in order to obtain more accurate results and interpretation of the shear strength of the soil, a longer soaking period might be the best option since previous results shows that shear strength of soil is keep decreasing with time if the sample is being soaking for too long.

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